

UChile - LMMG 2015



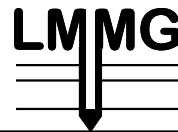
Shear Wave Velocity (V_s): Measurement, Uncertainty, and Utility in Seismic Hazard Analysis

Robb Eric S. Moss, Ph.D., P.E.

Assoc. Prof. of Earthquake, Geotechnical, and Risk Engineering



CAL POLY



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

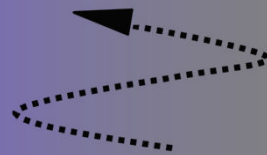
DEFINITIONS:

What is V_s and why is it of interest in earthquake engineering.

APPLICATIONS:

How V_s is used in earthquake engineering for:

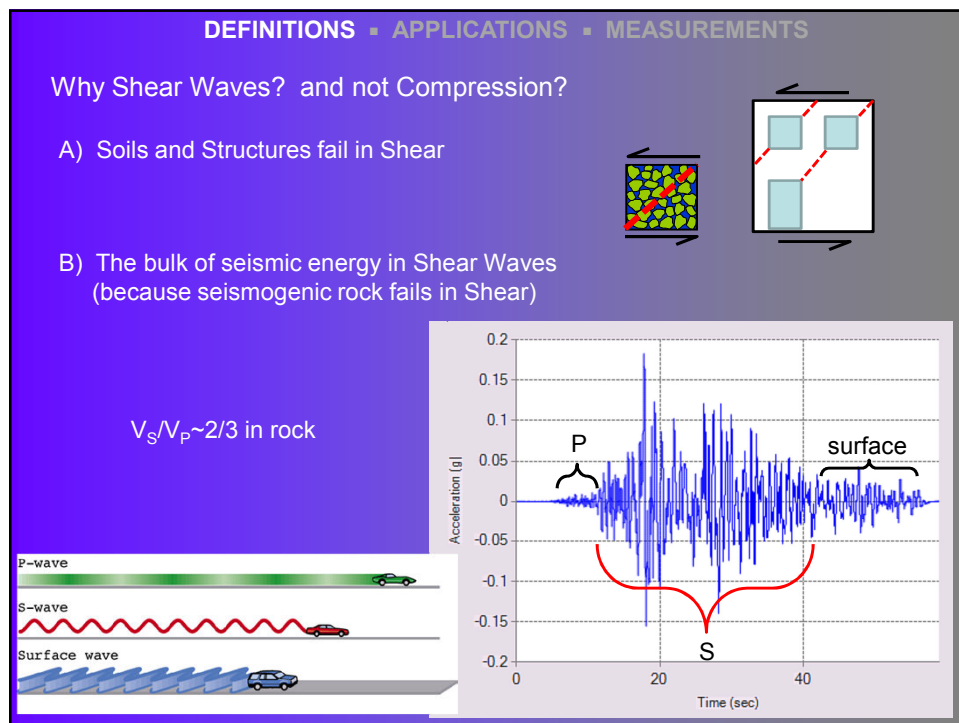
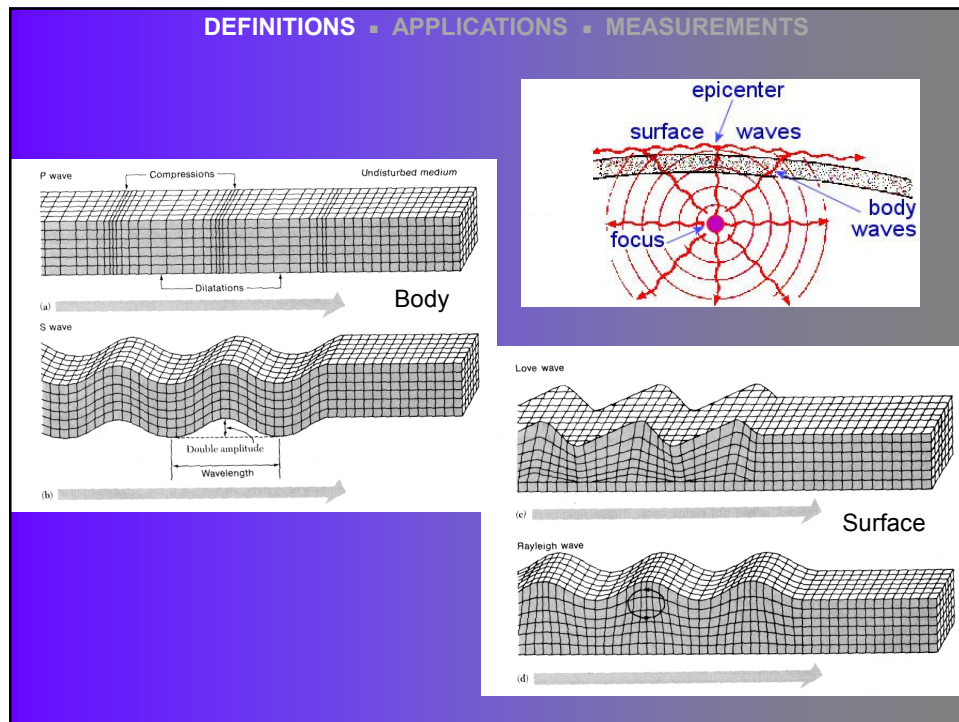
- Site Response
- Liquefaction
- Dynamic Slope Stability
- Fault Rupture
- Other



MEASUREMENTS:

How V_s is measured in the lab and in the field.

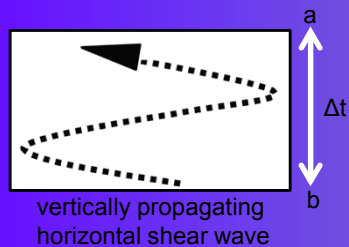
- Field methods: invasive and noninvasive
- What uncertainty exist in the various measurements
- How this uncertainty propagates in earthquake engineering



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

$$\text{Wave Velocity (m/s)} = 1 / \text{Slowness (s/m)}$$

$$= \text{Distance (m)} / \text{Travel time (s)}$$



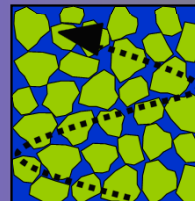
Geomaterial	V_s (m/s)
Peaty/Organic	20-80
Loose Sands – Soft Clays	100-200
Dense Sands –Stiff Clays	200-400
Soft Rock	450-800
Intact Shallow Rock	800-2000
Crustal Rock	2500-3500

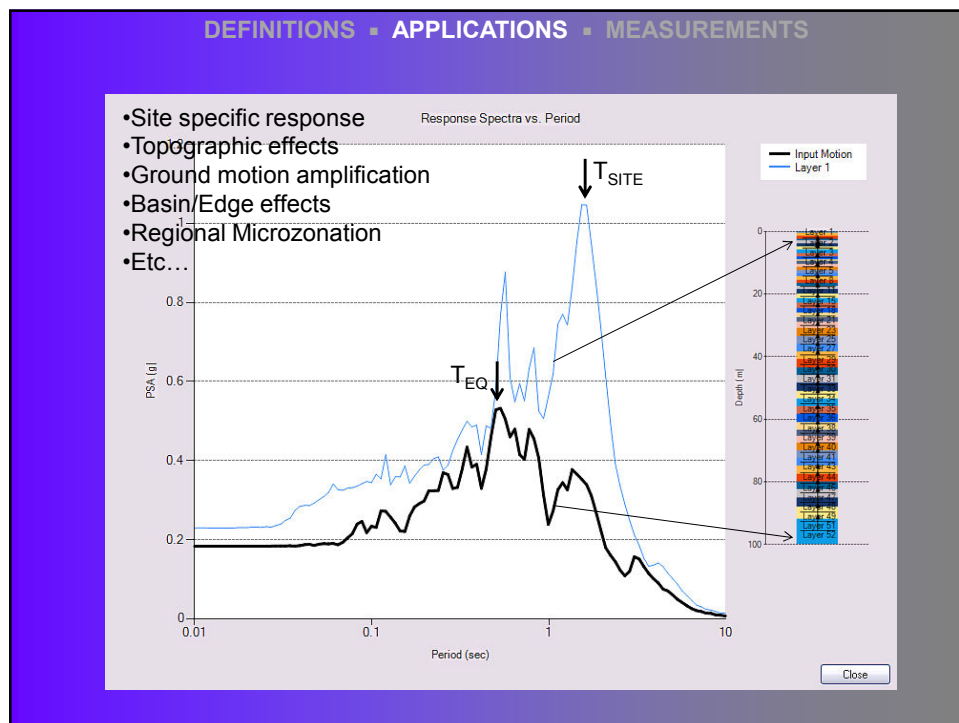
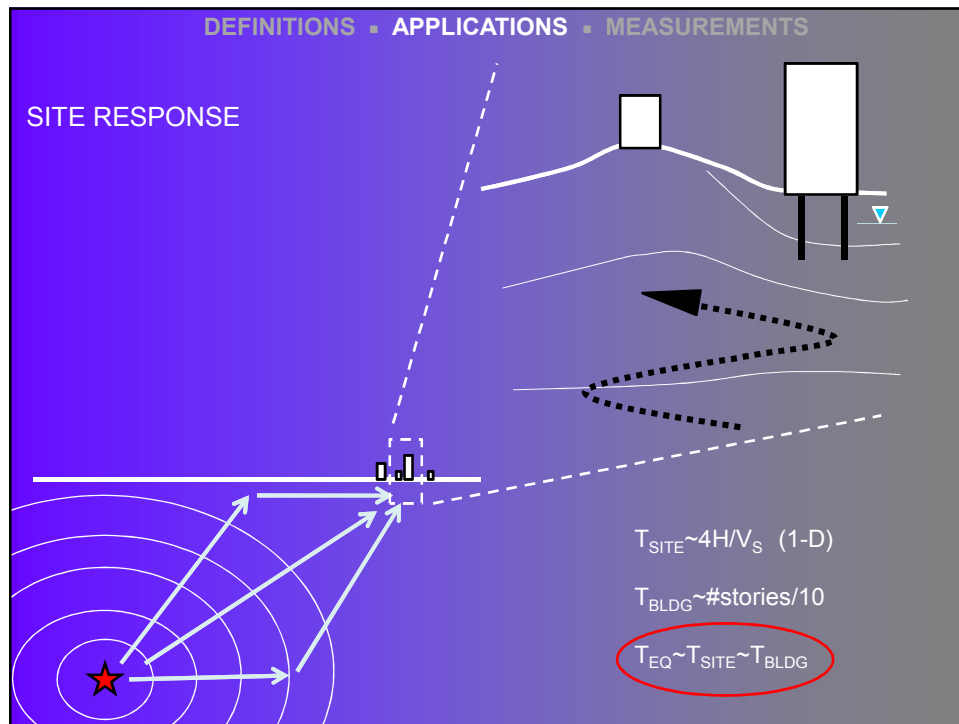
DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

$$G_o = \frac{\tau}{\gamma} = \rho \cdot V_s^2$$

The small strain shear stiffness is the ratio of the initial shear stress to the shear strain, equivalent to the product of the density of the soil and the squared shear wave velocity.

- V_s in soil is a function of:
- individual particle stiffness
 - bulk matrix stiffness
 - particle packing/arrangement
 - bonding or cementation agents
 - viscosity of the pore fluid
 - other properties





DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

V_s used as a single index parameter for estimating site response

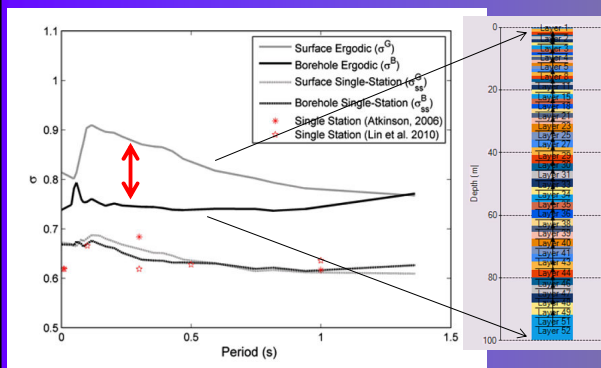
Code-based

NEHRP Category	Description
A	Hard rock; $V_{s30} > 1500$ m/s
B	Rock; $760 \text{ m/s} < V_{s30} \leq 1500 \text{ m/s}$
C	Very dense soil and soft rock; $360 \text{ m/s} < V_{s30} \leq 760 \text{ m/s}$
D	Stiff soil; $180 \text{ m/s} \leq V_{s30} \leq 360 \text{ m/s}$
E	A soil profile with $V_{s30} < 180$ m/s, or any profile with more than 3 m of soft clay defined as soil with $P_I > 20$, $w \geq 40$ percent, and average undrained shear strength in top 30m < 25 kPa
F	1. Soils vulnerable to potential failure or collapse under seismic loading such as liquefiable soils, quick and highly sensitive clays, collapsible weakly cemented soils. 2. Peat and/or highly organic clays with a thickness > 3 m, 3. Very high plasticity clays (Thickness > 8 m with Plasticity Index > 75) 4. Very thick, soft/medium stiff clays (Thickness > 36 m with average undrained shear strength in top 30 m < 50 kPa.)

Regression-based

$$\ln(Y) = f(M_w, R, V_{s30}, \dots) + \sigma_{\text{inter}} + \sigma_{\text{intra}}$$

DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS



V_s as a proxy for site response
-good in concept

V_{s30} as a single parameter
-incomplete at best

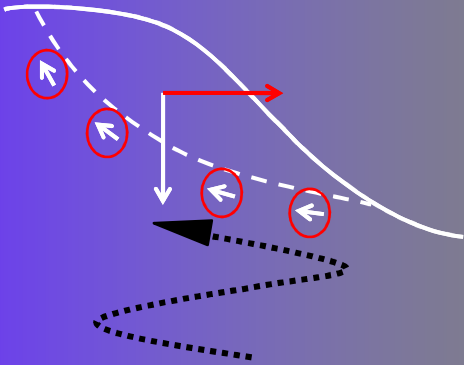
When forecasting ground motions for design the magnitude of the standard deviation can make a big difference in hazard and cost.

"...the large difference in standard deviations between surface and borehole data when using the ergodic assumption can be attributed to a poor parameterization of site response."

Rodriguez-Marek, Montalva, Cotton, & Bonilla (2011) BSSA

DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

DYNAMIC SLOPE STABILITY

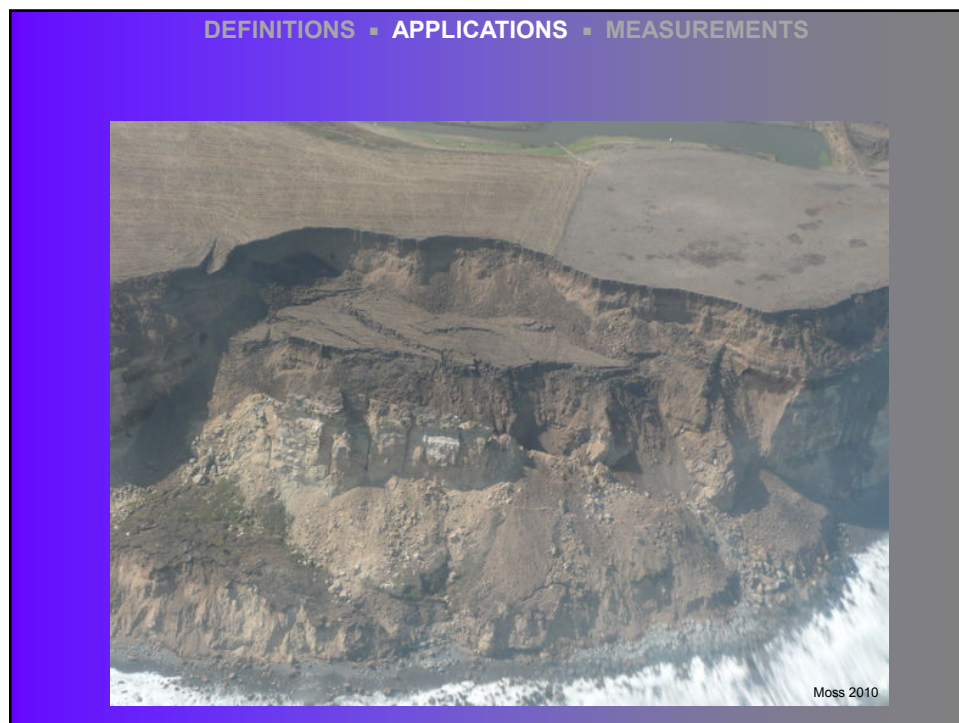


Primary Variables:

- a) Period of slide mass
- b) Period of earthquake
- c) Earthquake Duration
- d) Shear Strength

resonance

$$T_{SLIDE} \sim T_{EQ}$$

$$T_{SLIDE} \sim 4H/V_s (1-D)$$


DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

$$T_{\text{SLIDE}} \sim T_{\text{EQ}}$$

Resonance or First-Mode response results in “Newmark” (aka Ambraseys) type deformations.

Rigid-block /ductile deformation analysis is a function of V_s of slide mass.

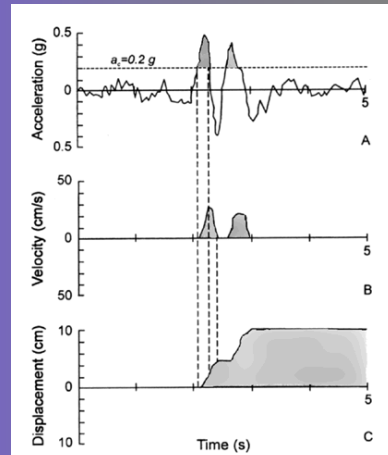
Makdisi & Seed (1978) JGE

Bray et al. (1998) JGGE

Bray and Travararou (2007) JGGE

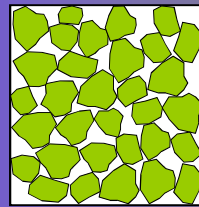
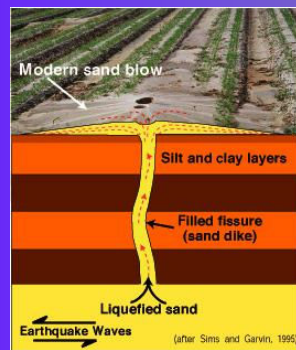
Jibson & Jibson (2003) USGS

Jibson et al. (2013) USGS

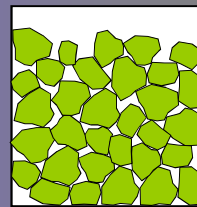


DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

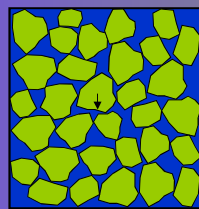
LIQUEFACTION



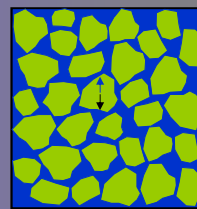
Dry Loose Sand



after Shaking

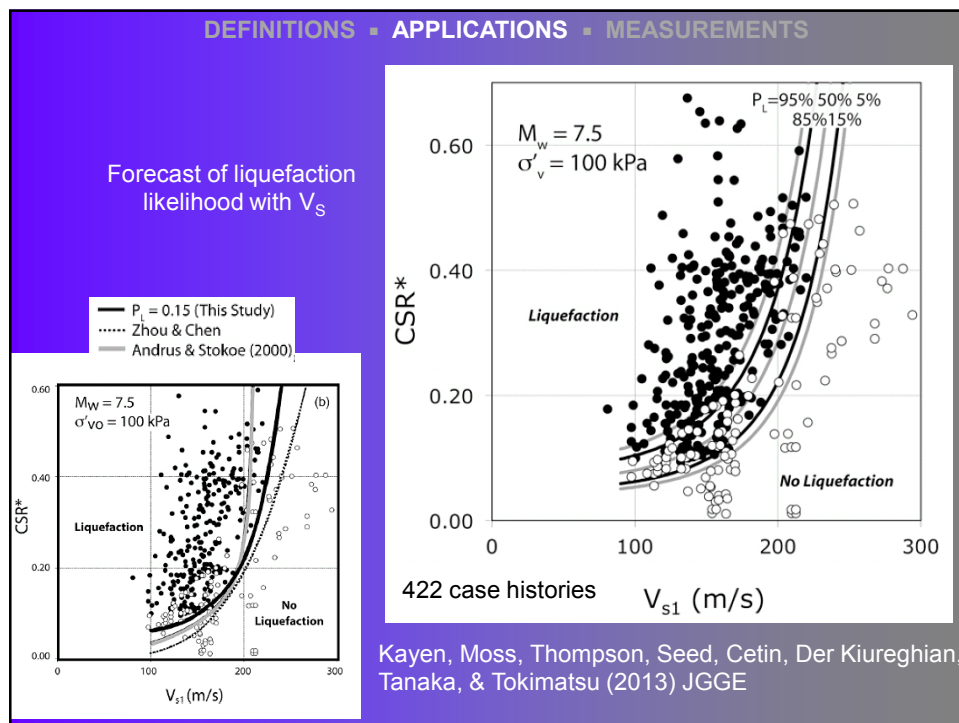


Saturated Loose Sand



after Shaking

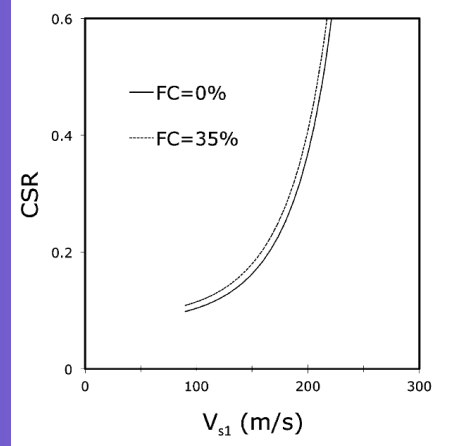
$$\tau_{ff} = \underbrace{c}_{0} + \underbrace{\sigma'_n}_{0} \tan \phi'$$



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Comments on V_s and Liquefaction Triggering

- Not good for assessing the influence of fines content (FC)
- Very good for assessing ageing (Holocene vs Pleistocene)
- Often very complimentary to CPT and/or SPT triggering analysis



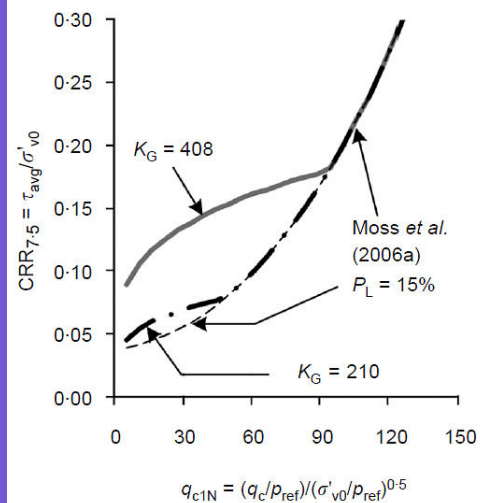
DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Ageing and Liquefaction

$$K_G = \frac{G_0/q_c}{\left[(q_c/p_{ref}) / (\sigma'_{v0}/p_{ref})^{0.5} \right]^{-m}} = \frac{G_0/q_c}{q_{c1N}^{-m}}$$

Older Soil = Higher V_s
 = Higher G_0
 = Higher CRR

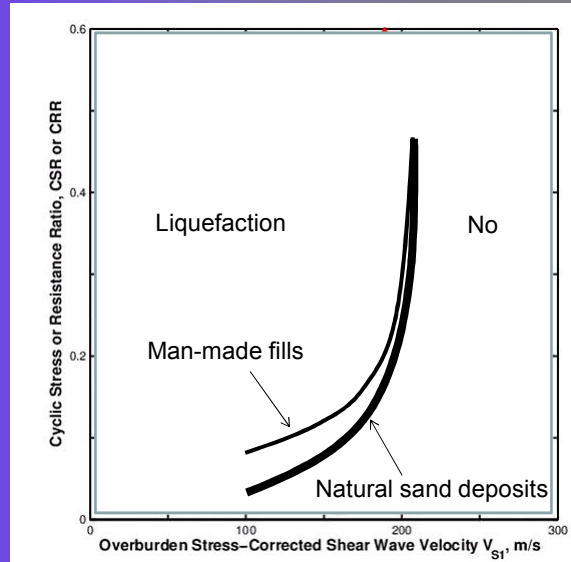
Schneider and Moss (2011) Geotechnique



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

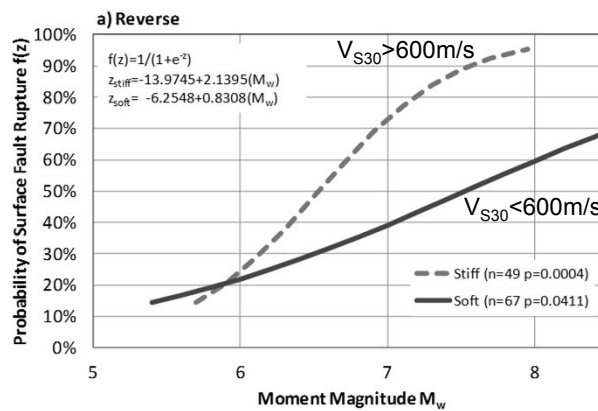
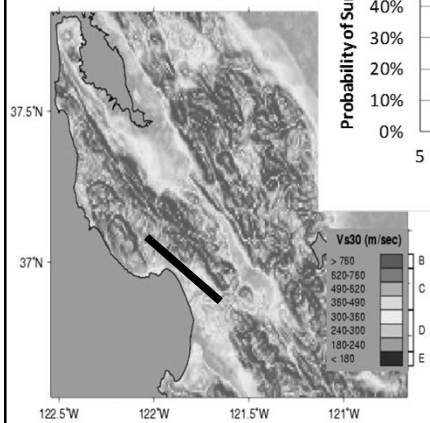
Dobry et al. (2014) JGGE

Ageing and Liquefaction

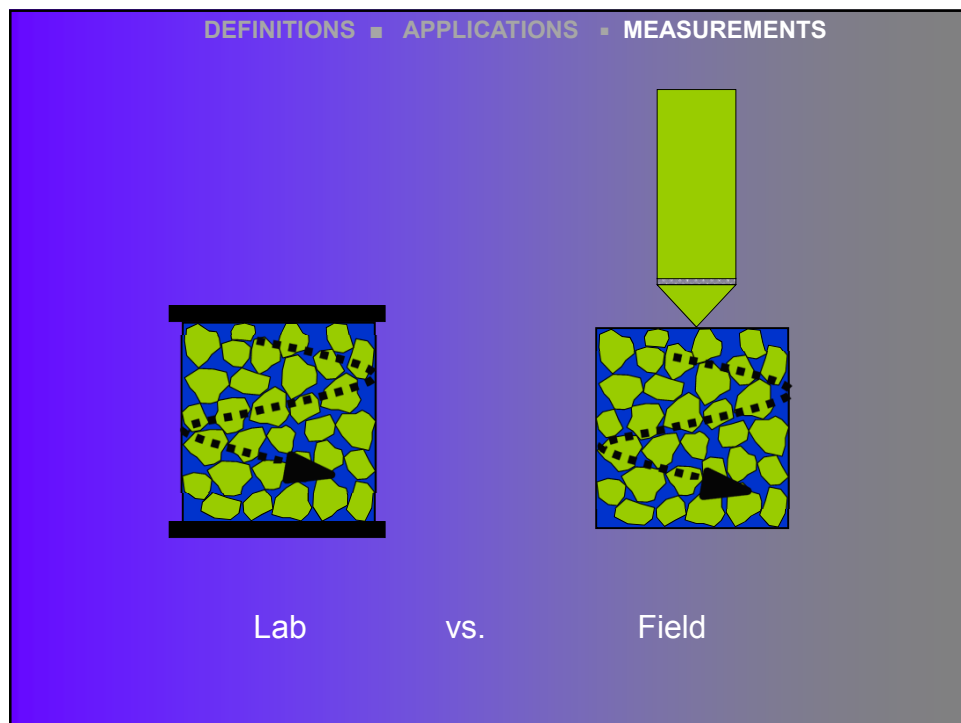
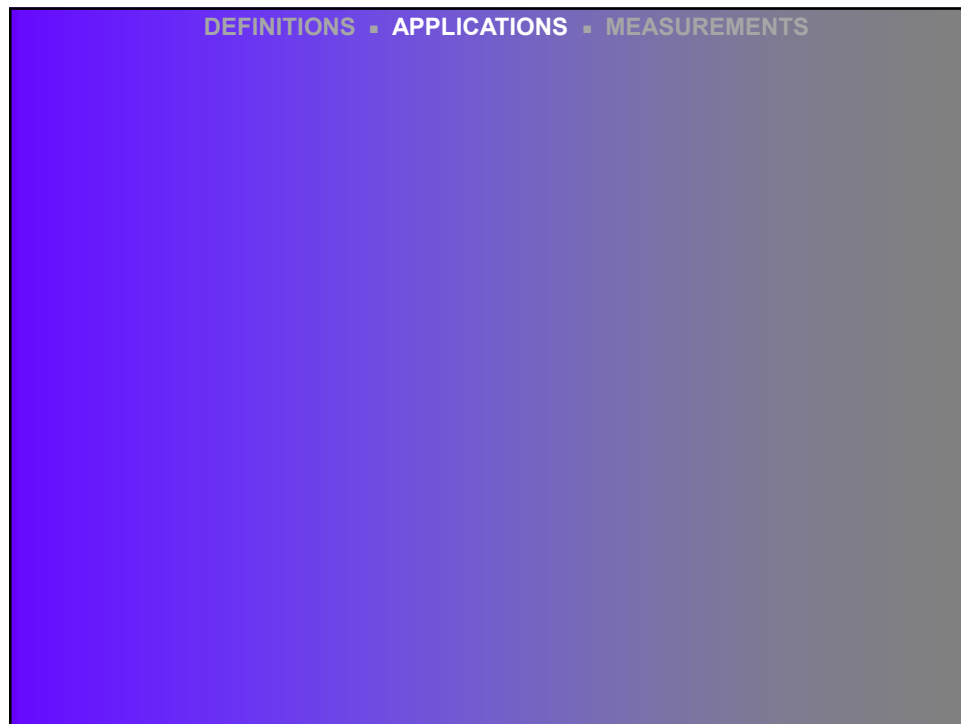


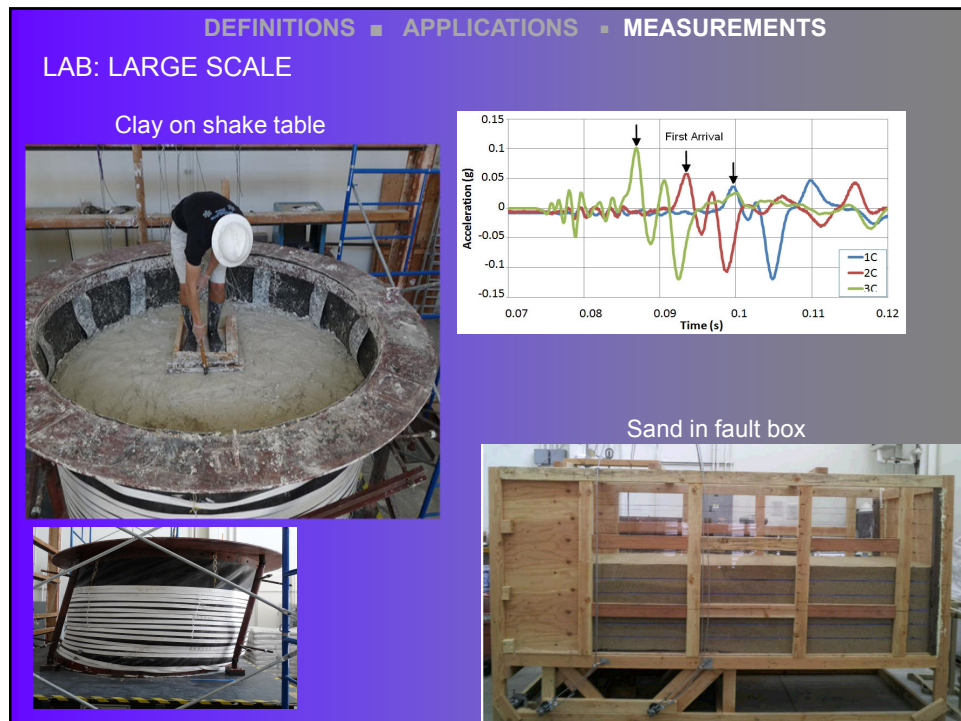
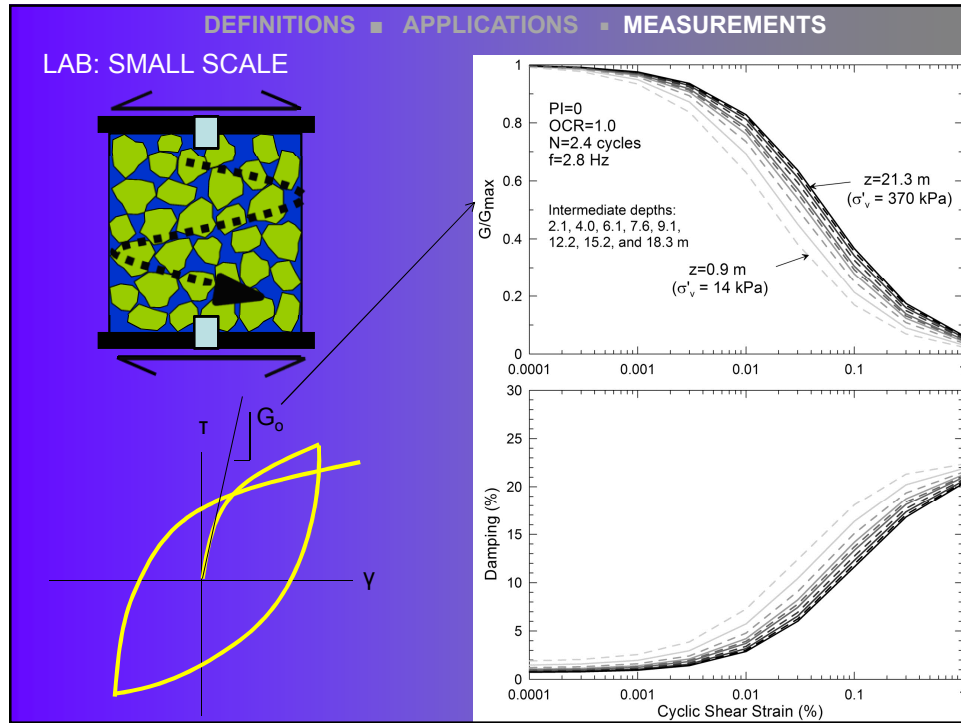
DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

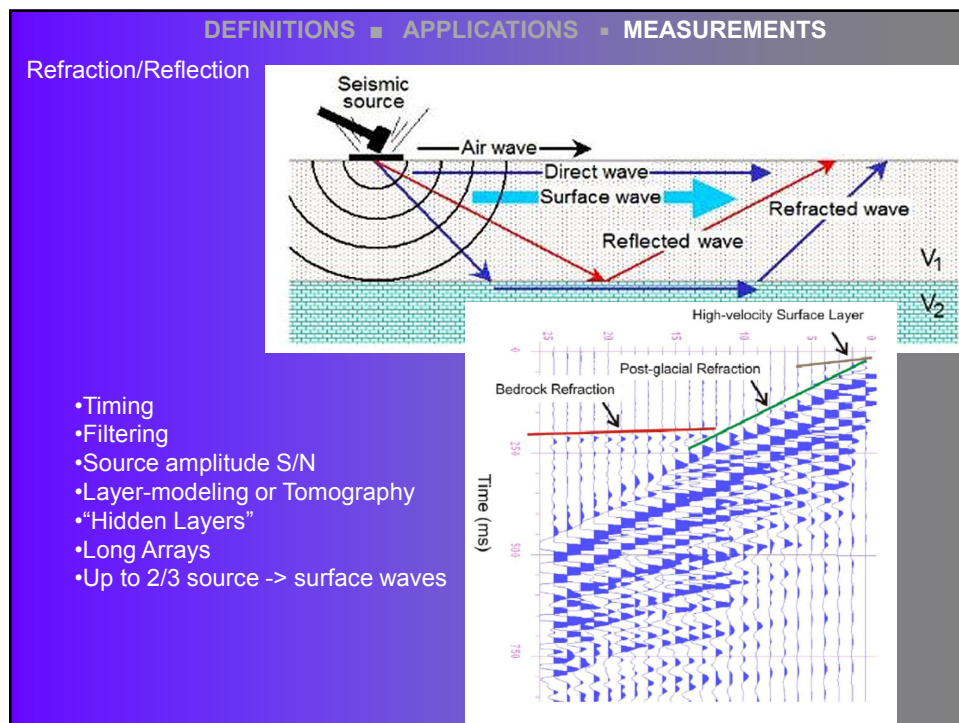
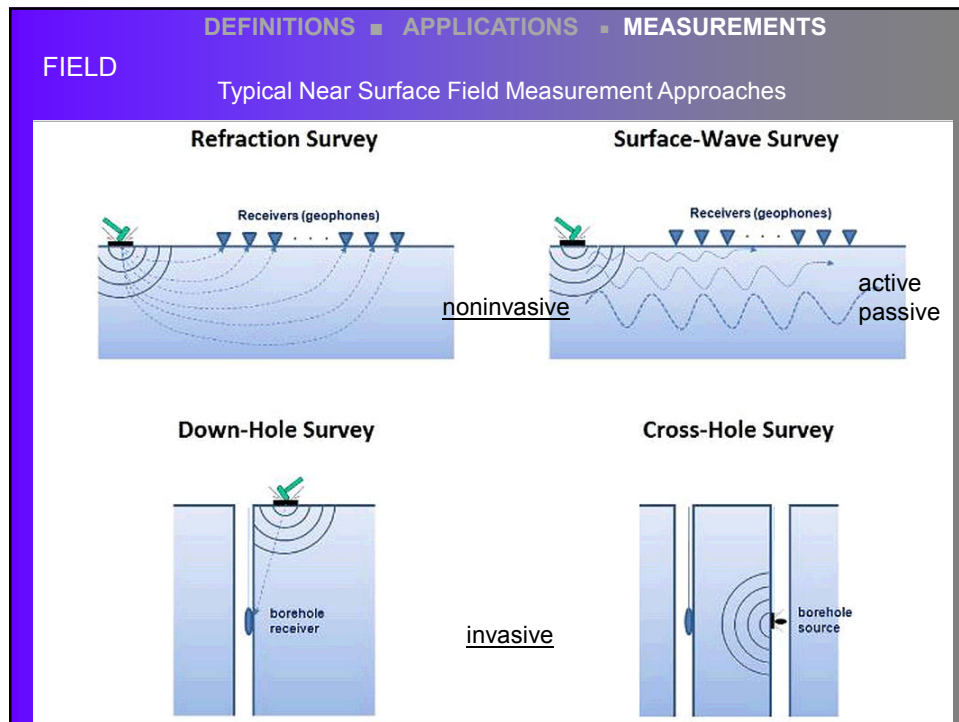
SURFACE FAULT RUPTURE



Moss et al. (2013) SRL

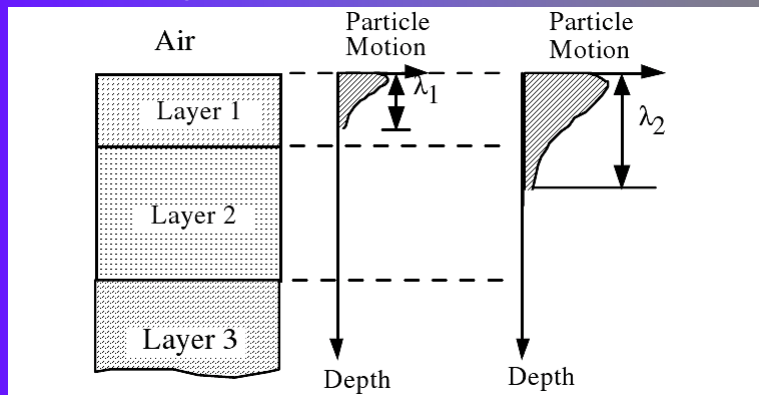






DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing



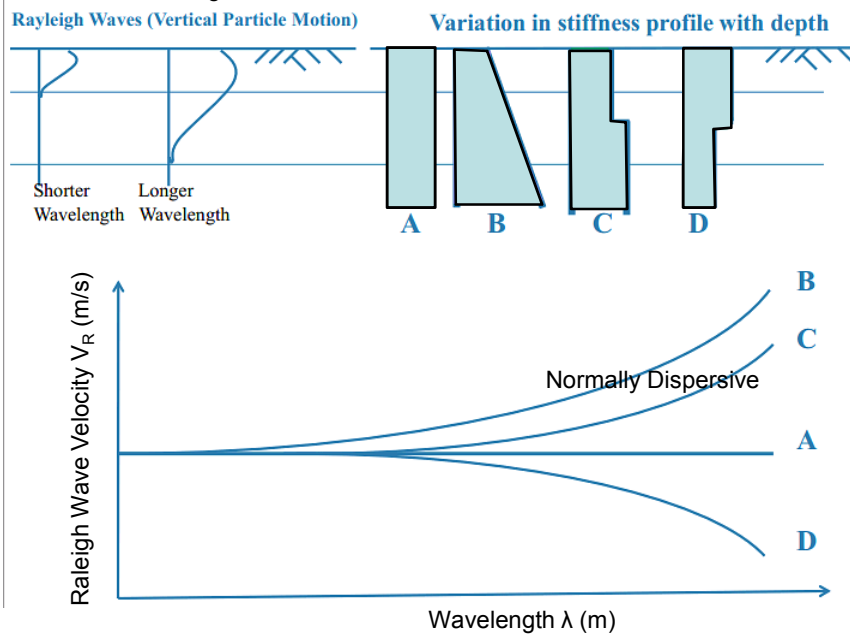
• Rayleigh and Love waves are **dispersive**; different wavelengths travel at different velocities based on the velocity of the materials they encounter (Aki and Richards, 2003).

• Longer wavelengths travel through deeper layers (depth $\sim 3\lambda_R$).

$$\begin{aligned} V_R &= 0.95(V_S) \quad \text{for } \nu = 0.5 \\ &= 0.85(V_S) \quad \text{for } \nu = 0.0 \end{aligned}$$

DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing

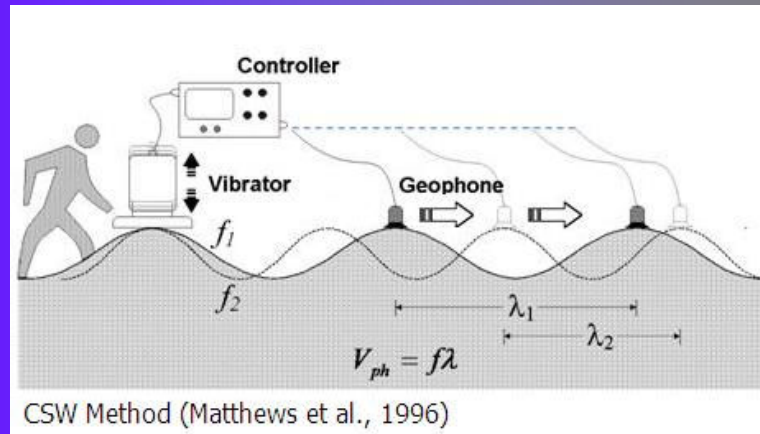


DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing

Steady State Method

Van der Pol (1951), Jones (1955)



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing

SASW, Stokoe and Nazarian 1985

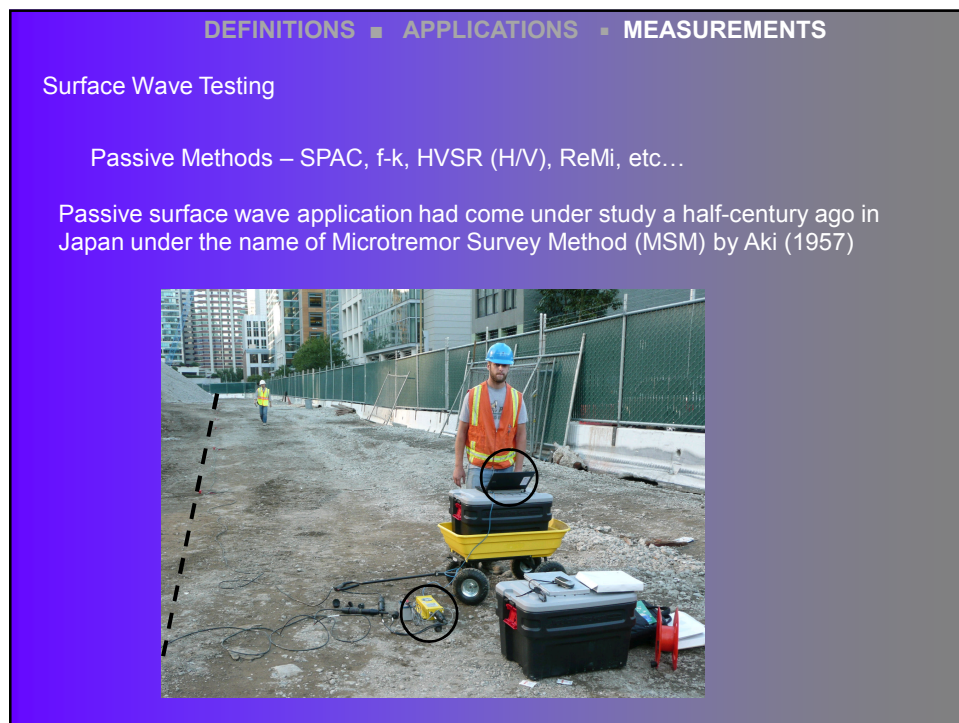
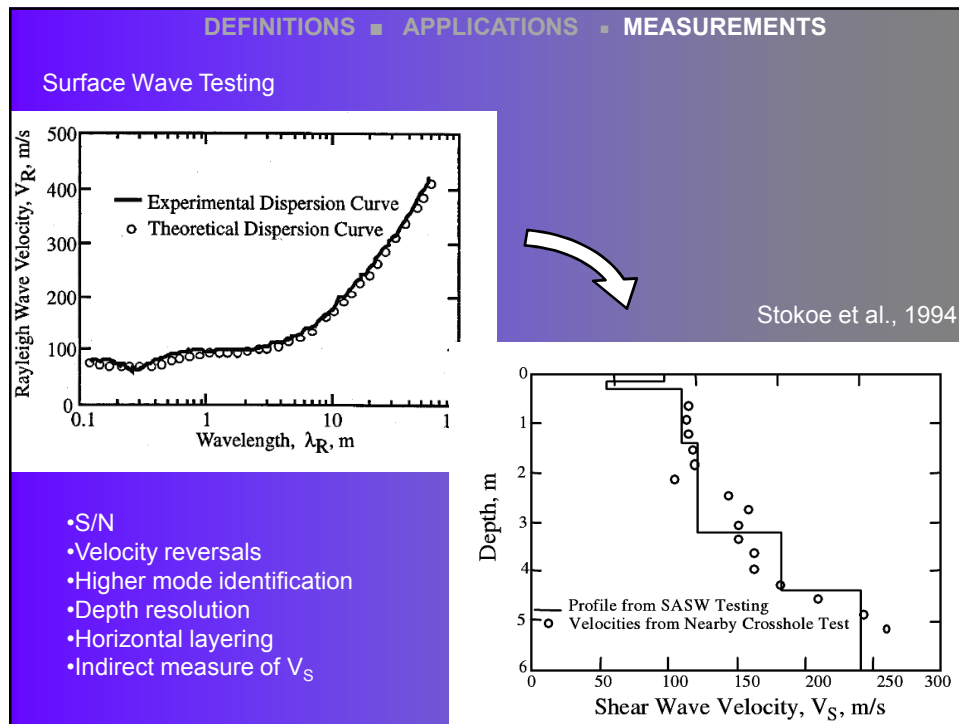
MASW, Park et al., 1994

automated steady state



GEER 2011 Tohoku





DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing

Passive -> same caveats as SASW/MASW and....

H/V

- Assumes that S is influenced by site conditions but P is not
- Need more info to estimate V_s profile

SPAC

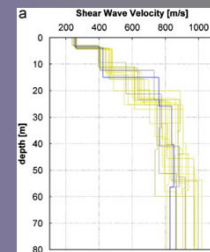
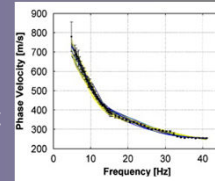
- Azimuthal coverage needed (circular array or similar)

f-k

- Generally requires higher ambient energy with particular orientation

ReMi

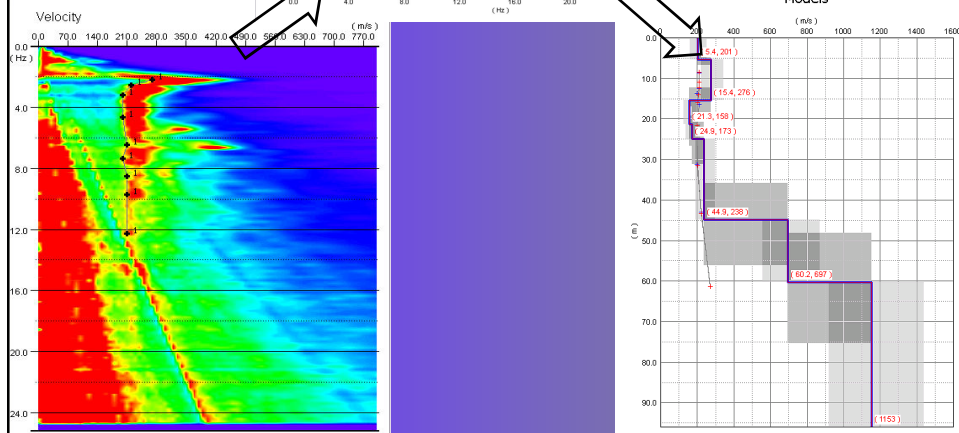
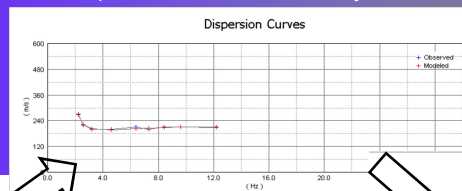
- Linear array orientation with respect to ambient noise



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Surface Wave Testing

Dispersion Curve -> Velocity Profile



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

PRECISION AND ACCURACY

What methods work best? Under what conditions?

Variables that influence Precision :

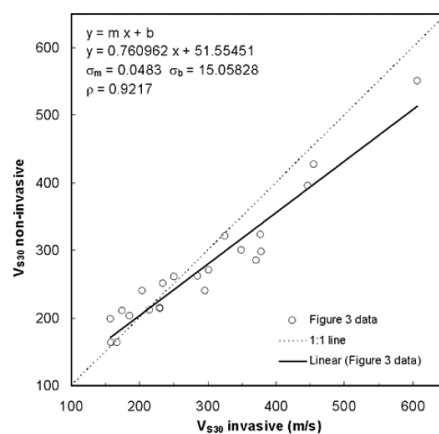
- Equipment
- Methods
- Site-conditions
- Operator experience
- other

Method	COV=stdev/mean
Invasive (DH, SCPT, P-S Log, CH)	1-3% (?)
SASW and MASW	5-6%
ReMi	5-15%
f-k and SPAC	?
Inferred (Geology or Topography)	10-50%

Moss (2008) BSSA

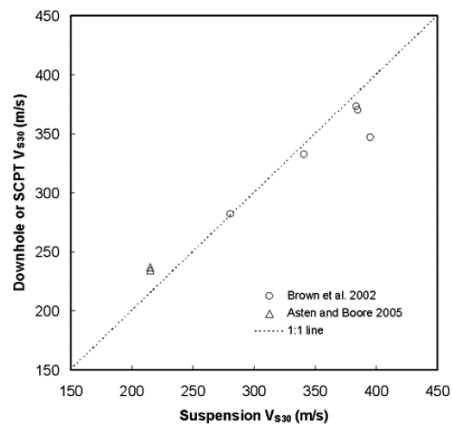
DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

PRECISION AND ACCURACY



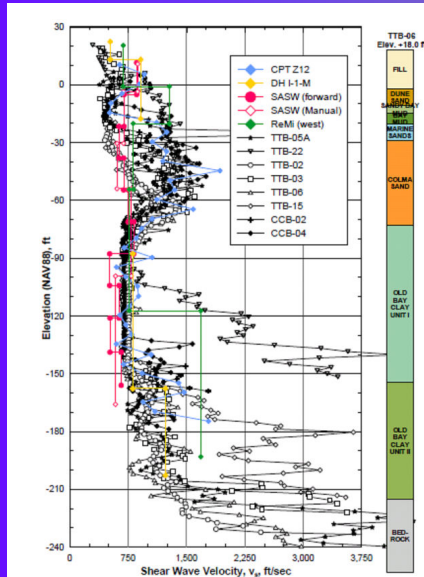
Accuracy of invasive a function of soil disturbance...
 Accuracy of noninvasive a function of resolution /fidelity of surface waves...

Moss (2008) BSSA



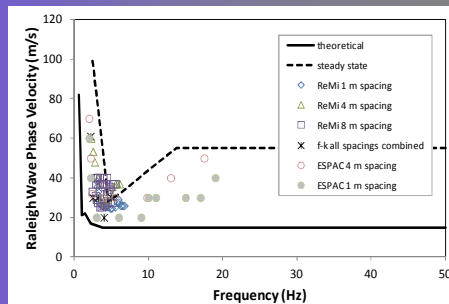
DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

PRECISION AND ACCURACY



Blind Studies; e.g.,

- Asten and Boore (2005)
- Brown, Boore, & Stokoe (2002),
- Xia et al., (2002)
- Other...
- more needed!



DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

PRECISION AND ACCURACY

How accurate is accurate enough? Depends...

Site Response

- need resonant period of entire soil column
- ~ $\Delta 10\%$ does not influence results appreciable
- rough estimate excludes period lengthening and structural softening

Liquefaction

- need V_s of specific layer
- ~ $\Delta 10\%$ does influence results
- uncertainty could result in large cost difference: Mitigation vs. non-Mitigation

Not-Important



Important

DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS

Future Work:

- Need more blind study sites with as many methods as possible
- Focus on difficult sites: dense urban, stiff-over-soft, shallow rock,...
- Establish specific minimum requirements for different methods
- Consensus best-practices because there is no single best-method
- Measure V_s at every site, for every project:
Small strain stiffness is the easiest engineering property to directly measure
- Better measure of V_s = more accurate seismic hazard analysis

Thank You

DEFINITIONS ■ APPLICATIONS ■ MEASUREMENTS**Shear Wave Velocity (V_s): Measurement, Uncertainty, and Utility in Seismic Hazard Analysis****By Robb Moss, PhD PE**

Shear wave velocity (V_s) is a primary input variable in many seismic hazard analyses. It is essential for performing seismic site response analysis, used as an index for liquefaction triggering analysis, important for identifying the modal resonance of seismic slopes and embankments, and has many other seismic hazard uses. Shear wave velocity can be measured both in the lab and in the field providing a crucial bridge between table-top and field scales. The measurement and determination of V_s can be accomplished with many methods, which range from conceptually straightforward using invasive methods to often conceptually and computationally complex using non-invasive methods. This presentation will cover the use of V_s in geotechnical earthquake engineering, discuss the uncertainty in the measurement of this property, and provide an overview of current research into quantifying the uncertainty, improving the methods, and applying the knowledge gained to reduce seismic hazards.